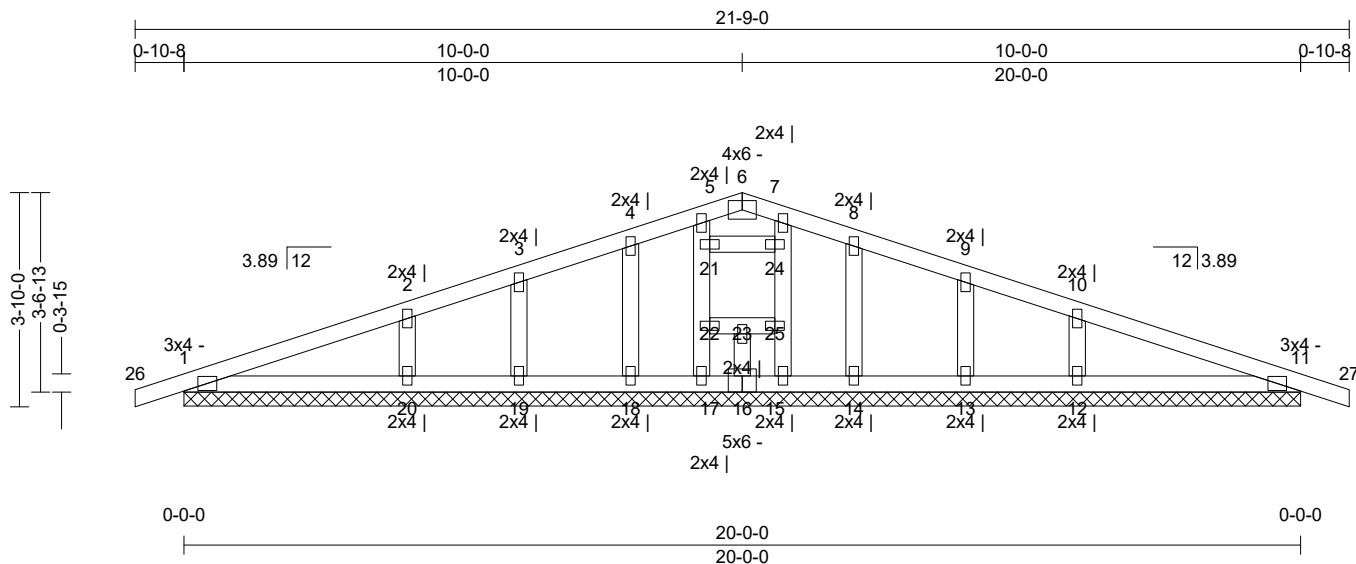


TUFF SHED
1777 SOUTH HARRISON STREET
SUITE 600
DENVER, CO 80210

TrussL01
Job: 520-1665567-FRED JMG
Design: FB
Date: 06/08/21 08:59:55
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3.89/12	2	0-10-8	0-10-8	0-0-0	0-0-0	1	24 in	71 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: FBC 2020/	TC: 0.15 (10-11)	Vert TL: 0 in	L/999	(11-12)	L/180
TCDL: 10	TPI 1-2014	BC: 0.05 (20-1)	Vert LL: 0 in	L/999	11	L/240
BCCL: 0	Rep Mbr: No	Web: 0.04 (2-20)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		252 lbs	87 plf	-16 lbs	-140 lbs	-241 lbs	-241 lbs	364 lbs

Material

TC: SPF #2 2 x 4
BC: SPF #2 2 x 4
Web: SPF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 160 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 20 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.150	-315 lbs
	10-11	0.150	-315 lbs
BC			
Web			

Notes

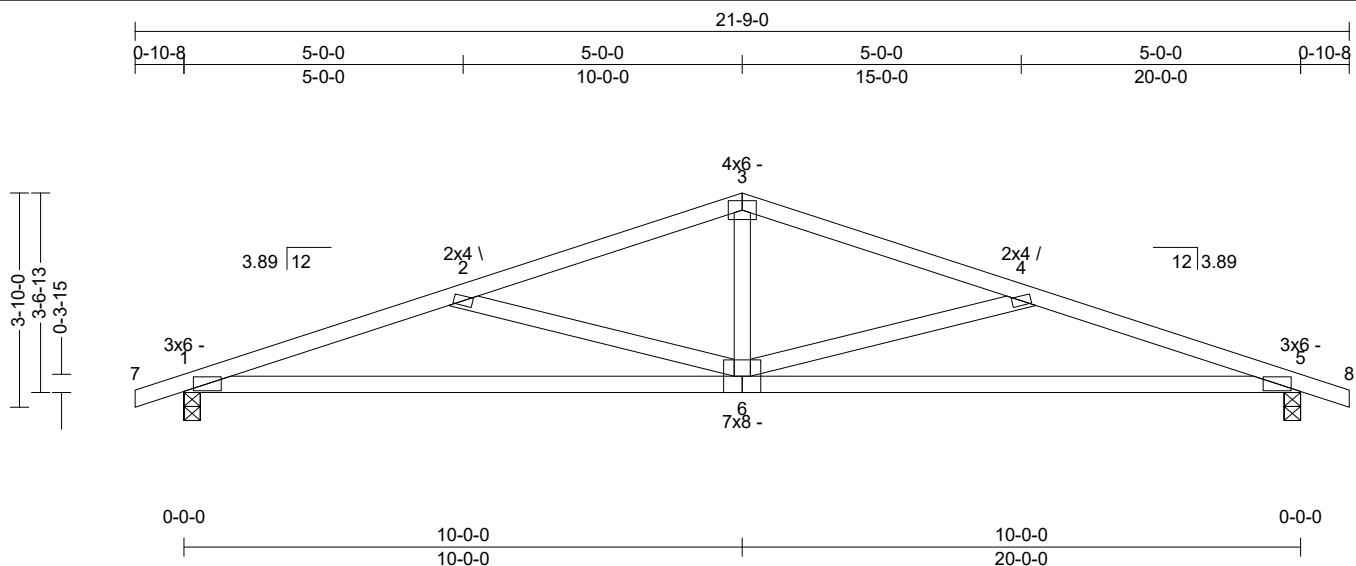
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 0% (Cq = 1.00).
- 7) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 8) A creep factor of 1.50 has been applied for this truss analysis.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 11, 1 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

7/21/2021

TUFF SHED
1777 SOUTH HARRISON STREET
SUITE 600
DENVER, CO 80210

TrussT01
Job: 520-1665567-FRED JMG
Design: JFB
Date: 06/08/21 09:00:26
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3.89/12	11	0-10-8	0-10-8	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : FBC 2020/	TC : 0.68 (2-3)	Vert TL: 0.4 in	L/576	(5-6)	L/180
TCDL : 10	TPI 1-2014	BC : 0.82 (5-6)	Vert LL: 0.15 in UP	L/999	(5-6)	L/240
BCLL : 0	Rep Mbr : Yes	Web : 0.29 (4-6)	Horz TL: 0.07 in		5	
BCDL : 10	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	852 lbs	-	-466 lbs	-877 lbs	-877 lbs	-28 lbs
5	1	3.5 in	1.50 in	852 lbs	-	-466 lbs	-877 lbs	-877 lbs	-

Material

TC: SPF #2 2 x 4
BC: SPF #2 2 x 4
Web: SPF #2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 4-7-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 160 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 20 ft x 24 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.530	-1,955 lbs	3-4	0.679	-1,430 lbs			
	2-3	0.679	-1,430 lbs	4-5	0.530	-1,955 lbs			
BC	5-6	0.815	1,838 lbs (-1,683 lbs)	6-1	0.815	1,838 lbs (-1,683 lbs)			
Web	2-6	0.288	689 lbs (-581 lbs)	3-6	0.129	526 lbs (-284 lbs)	4-6	0.288	689 lbs (-581 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 0 % (Cq = 1.00).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.50 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.

7/21/2021