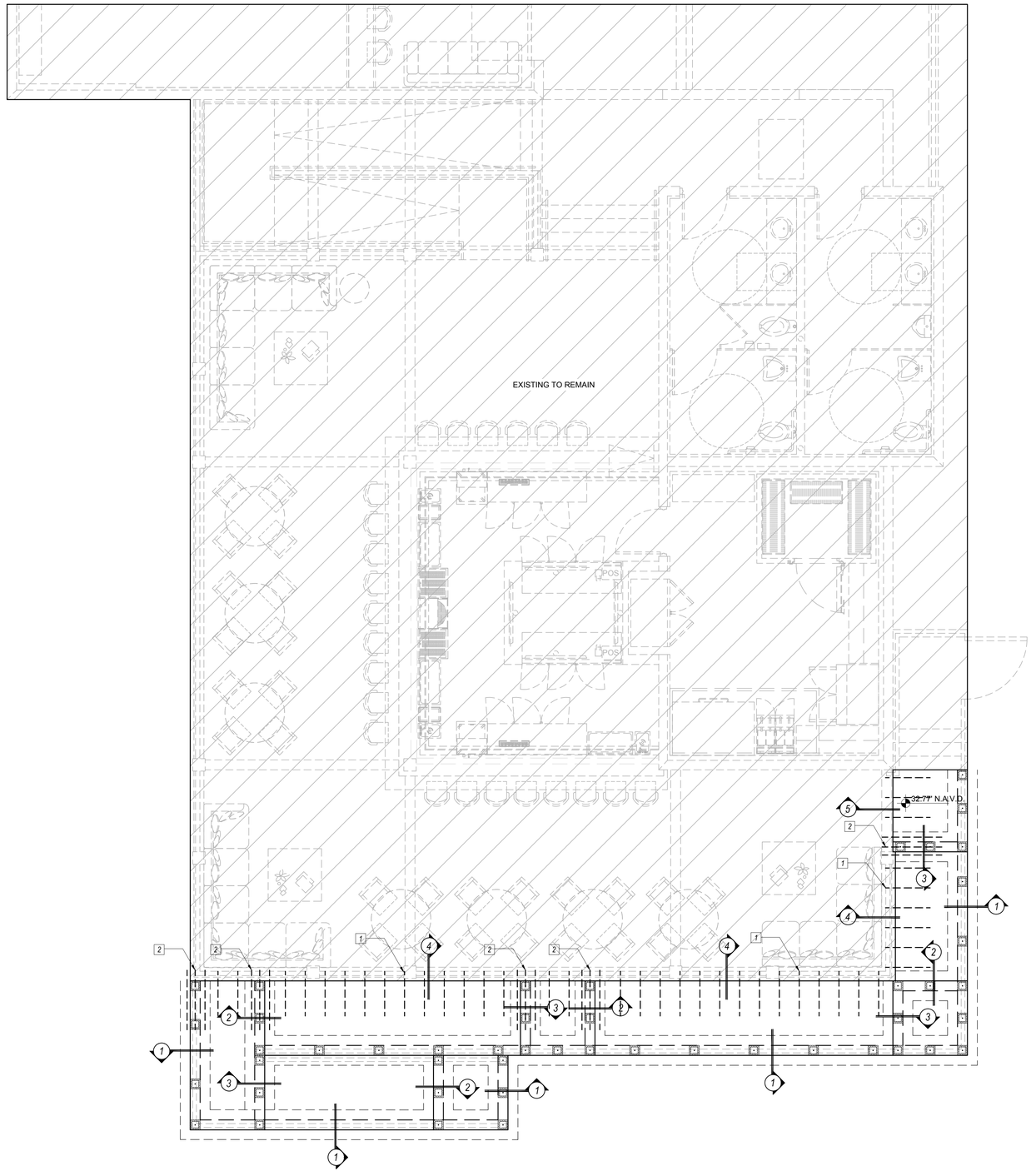


FILLED CELL LEGEND	
	8" REINFORCED MASONRY BLOCK WITH (1) - #5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
	12" REINFORCED MASONRY BLOCK WITH (1)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
	16" REINFORCED MASONRY BLOCK WITH (2)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
	8" - 45° REINFORCED MASONRY CORNER BLOCK WITH (2)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE - (BUTTERFLY TYPE)

NOTE:
COORDINATE ALL COLUMN AND PAD FOOTING LOCATIONS WITH ARCHITECTURAL WALL DIMENSION PLANS

NOTE:
COORDINATE ALL RECESSES FOR DOOR THRESHOLDS WITH ARCHITECTURAL PLANS AND MANUFACTURERS REQUIREMENTS.

FOOTING PAD SCHEDULE	
F.PAD	FOOTING PAD DESCRIPTION
FP-1	36"X36"X12" CONC. FOOTING PAD WITH (4) #5 EACH WAY 3" CLEAR FROM BOTTOM OF PAD
FP-2	48"X48"X16" CONC. FOOTING PAD WITH (6) #5 EACH WAY 3" CLEAR FROM BOTTOM OF PAD

FOUNDATION NOTES:

1. NOTE 1:
DOWEL NEW SLAB TO EXISTING SLAB WITH (1) ROW 36" LONG #5 REBAR AT 16" O.C. PLACED 2 1/2" BFF. AND EPOXY SET REBAR WITH A MIN. 6" EMBEDMENT.

2. NOTE 2:
EPOXY SET NEW GRADE BEAM TO EXISTING WITH (9) 48" LONG #6 REBAR IN (3) ROWS. FIRST ROW AT 4" BFF, SECOND ROW 8" BFF AND THIRD ROW AT 12" BFF. WITH A MIN. 12" EMBEDMENT

FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION PLAN

DATE: 09/10/21 | PROJ. NO.: 1685-52-02 | DWN. BY: FAA

NO.	REVISION	DATE

I CERTIFY THAT THESE PLANS AND SPECIFICATIONS CONFORM TO LOCAL BUILDING CODE REQUIREMENTS

Derek Cronin — Digitally signed by Derek Cronin — Date: 2021.09.16 14:25:54-04'07'

DEREK F. CRONIN — LICENSED PROFESSIONAL ENGINEER

CRONIN ENGINEERING, INC.
CERTIFICATE OF AUTHORIZATION NUMBER: 8597
6627 WILLOW PARK DRIVE
NAPLES, FL 34109
PHONE: (239) 593-2157 FAX: 593-8820

POPSTROKE PORT ST. LUCIE
SARASOTA, FL

MHK ARCHITECTURE & PLANNING
NAPLES, FL

S-1

NOTES:

1. COORDINATION OF CONSTRUCTION INCLUDING VERIFICATION THE RESPONSIBILITY OF THE CONTRACTOR. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONSTRUCTION. FOR DETAILS AND DIMENSIONS NOT SHOWN SEE ARCHITECTURAL DRAWINGS.
2. RECESSES AND CURBS FOR DOORS ARE NOT SHOWN. REFER TO ARCHITECTURAL FLOOR PLAN FOR SIZE AND LOCATION.
3. REINFORCING STEEL SHALL BE ASTM A615, GRADE 60 DEFORMED BARS, FREE FROM OIL SCALE AND RUST. LAP SPLICES SHALL BE 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED.
3. ALL CONCRETE SHALL OBTAIN A COMPRESSIVE STRENGTH OF 4000 P.S.I. IN 28 DAYS, AND SHALL CONFORM TO THE REQUIREMENTS OF ACI 318-14.
4. CONCRETE COVER REQUIREMENTS FOR REINFORCING STEEL
   (A) -CONCRETE CAST AGAINST EARTH SHALL HAVE A MINIMUM CLEAR COVER OF 3" OVER REINFORCING STEEL.
   (B) -CONCRETE EXPOSED TO EARTH OR WEATHER SHALL HAVE A CLEAR COVER OF 1 1/2" OVER #5 REBARS OR SMALLER, AND 2" FOR REBARS #6 OR LARGER.
   (C) -CONCRETE SLABS WITH EXTERIOR EXPOSURE SHALL HAVE A CLEAR COVER OF 1 1/2" OVER REINFORCING STEEL. INTERIOR CONCRETE SLABS SHALL HAVE A MINIMUM CLEAR COVER OF 1" OVER REINFORCING STEEL. (NOTE: SLABS ON GRADE SHALL BE CAST ON A VAPOR BARRIER.)
   (D)-INTERIOR CONCRETE BEAMS REQUIRE 1 1/2" CLEAR COVER OVER REINFORCING STEEL.
5. FORM WORK SUPPORTING CONCRETE BEAMS, SLABS, ETC., MAY NOT BE REMOVED UNTIL THE CONCRETE HAS ATTAINED 80% OF THE DESIGN MINIMUM STRENGTH. DETERMINATION OF THE IN PLACE CONCRETE STRENGTH SHALL BE DETERMINED BY LABORATORY TESTING OF CONCRETE CYLINDER.
6. FORMS SHALL BE CLEAN FROM DEBRIS PRIOR TO PLACEMENT OF CONCRETE.
7. MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ACI 530-10/ASCE 5-10/TMS 402-10, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
8. MASONRY SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH (f'm') OF 1500 P.S.I.. GROUTED MASONRY CELLS SHALL BE FILLED WITH A GROUT THAT ACHIEVES A MINIMUM COMPRESSIVE STRENGTH OF 2000 P.S.I. AFTER 28 DAYS.
9. HORIZONTAL MASONRY WALL REINFORCING SHALL BE CONTINUOUS HORIZONTALLY ALONG A SPECIFIED COARSE OF MASONRY AND THROUGH CORNERS AND INTERSECTIONS IN THE WALL. HORIZONTAL REINFORCING SHALL BE PROVIDED FOR ALL MASONRY WALLS. USE 8 GAGE WIRE LADDER AT 16" O.C. UNLESS OTHERWISE NOTED
10. STRUCTURAL STEEL SHALL CONFORM TO ANSI/AISC 360-16, EXCEPT TUBULAR STEEL COLUMNS, WHICH ARE TO BE CONSTRUCTED TO 46 k.s.i. YIELD STRENGTH. ALL BOLTS SHALL BE A325 BOLTS UNLESS OTHERWISE NOTED. WELDS SHALL BE PERFORMED WITH A E70xx ELECTRODE.
11. ALL TIMBER MEMBERS SHALL BE CONSTRUCTED OF No.2 S.Y.P. UNLESS OTHERWISE NOTED ON DRAWINGS.
12. ALL LVL MEMBERS SHALL HAVE AN ALLOWABLE BENDING STRESS OF 2,750 p.s.i. AND AN ALLOWABLE SHEAR STRESS OF 250 p.s.i.
13. ALL WINDOW AND DOOR CERTIFICATIONS SHALL BE BY THE RESPECTIVE MANUFACTURER.
14. TRUSS DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.
15. IF THIS STRUCTURE IS TO BE LOCATED IN THE COASTAL FLOOD HAZARD ZONE, ALL ELECTRICAL AND MECHANICAL DEVICES SHALL BE LOCATED AT OR ABOVE THE FLOOD PLANE. THE FLOOD PLANE ELEVATION LABELED ON OUR DRAWINGS SHALL BE CONFIRMED BY A REGISTERED LAND SURVEYOR. WE TAKE NO RESPONSIBILITY IN THE DETERMINATION OF THIS ELEVATION.
16. CONTRACTOR TO PROVIDE AND FIELD LOCATE VENTILATION RELIEF OF HYDROSTATIC PRESSURE. PROVIDE MINIMUM 1 SQ. IN. OF VENTILATION PER 1 SQ. FT. GARAGE SLAB: INSTALL VENTS AT MAXIMUM OF 12" ABOVE FINISH GRADE.
17. IF NOT OTHERWISE SPECIFIED ALL FILL SHALL BE CLEAN COARSE SAND FREE OF ROOTS AND OTHER DELETERIOUS MATERIAL, FILL SHALL BE PLACED IN 12" LIFTS AND COMPACTED WITH A HEAVY VIBRATORY ROLLER TO 95% OF MAXIMUM MODIFIED PROCTOR DENSITY IN ACCORDANCE WITH ASTM 1557.
18. THE FOUNDATION WAS DESIGNED USING A NET ALLOWABLE SOIL BEARING CAPACITY OF 2,500 P.S.F. CONTRACTOR TO VERIFY.
19. BUILDER ALLOWED TO WET STICK DOWELS.

NOTE:
ROOF COVERING MATERIALS SHALL COMPLY WITH THE FBC 2020 7TH EDITION. FOLLOW MANUFACTURERS REQUIREMENTS FOR INSTALLATION.

WALL AND CEILING GYPSUM BOARD APPLICATION PER FBC 2020 7TH EDITION.

1. FOR TYPICAL INTERIOR WALLS 3/8" SINGLE-PLY GYPSUM BOARD NAIL WITH 4d NAILS AT 8" O.C. SHALL BE APPLIED TO ALL RESIDENCE FRAMING MEMBERS SPACED AT 16" O.C.
2. FOR GARAGE INTERIOR WALLS 1/2" SINGLE-PLY GYPSUM BOARD NAIL WITH 6d NAILS AT 8" O.C. SHALL BE APPLIED TO ALL GARAGE FRAMING MEMBERS SPACED AT 16" O.C.
3. FOR ALL CEILING FRAMING MEMBERS, APPLY 5/8" SINGLE-PLY GYPSUM BOARD OR 1/2" SAG RESISTANT GYPSUM BOARDS NAIL WITH 6d. NAILS AT 7" O.C. PERPENDICULAR TO CEILING FRAMING.
4. FOR GARAGE CEILINGS BENEATH HABITABLE ROOMS 5/8" TYPE-X GYPSUM BOARD SHALL BE APPLIED NAIL WITH 6d. NAILS AT 6" O.C. PERPENDICULAR TO CEILING FRAMING.
5. PER THE FBC 2020 7TH EDITION. INCLUDING EXTERIOR WALLS SEPARATED BY LESS THAN 6' SHALL NOT HAVE LESS THAN ONE-HOUR-FIRE-RESISTIVE RATING WITH EXPOSURE FROM BOTH SIDES OF WALLS.

PRECAST LINTEL DEPTH INCREASE NOTE

CONTRACTOR MAY CONSTRUCT LINTEL BEAMS DEEPER THAN SHOWN ON LINTEL BEAM PROFILES TO A MAXIMUM DEPTH OF 72". REINFORCING STEEL SHALL FOLLOW THE LINTEL BEAM PROFILE WITH THE BOTTOM BARS AND LINTELS PLACED AT THE FINAL BOTTOM DEPTH OF BEAM.

MOST BEAM DESIGNATIONS CALL FOR A 16" DEEP BEAM TO BE CAST MONOLITHICALLY WITH THE TIE BEAM ON TOP OF THE MASONRY WALLS. FOR OPENINGS REQUIRING A DEEPER HEADER PRECAST LINTELS MAY BE USED. THE LINTELS ARE TO BE REINFORCED WITH (2) #5 REBAR AND GROUTED SOLID.

ALL PRECAST LINTELS:

PRE-CAST LINTELS SHALL BE MANUFACTURED BY CASTCRETE. USE 8F8-1B/1T.

DRIP EDGE FASTENING

PER FBC 2020 7TH EDITION PROVIDE DRIP EDGE AT EAVES AND GABLES OF SHINGLE ROOFS. OVERLAP SHALL BE A MIN. OF 3". EAVE DRIP EDGES SHALL EXTEND 1/2" BELOW SHEATHING AND EXTEND BACK ON ROOF A MIN. OF 2". DRIP EDGES AT EAVES SHALL BE PERMITTED TO BE INSTALLED EITHER OVER OR UNDER THE UNDERLAYMENT. IF INSTALLED OVER UNDERLAYMENT, THERE SHALL BE A MIN. 4 1/2" WIDTH OF ROOF CEMENT INSTALLED OVER THE DRIP EDGE FLANGE. THE DRIP EDGE SHALL BE MECHANICALLY FASTENED A MAX. OF 4" O.C.

ROOF SHEATHING THICKNESS REQUIREMENTS

Rafter/Truss Spacing24 in. o.c.	WIND SPEED							
	115 mph	120 mph	130 mph	140 mph	150 mph	160 mph	170 mph	180 mph
Minimum Sheathing Thickness, inches(Panel Span Rating) Exposure B	7/16(24/16)	7/16(24/16)	7/16(24/16)	7/16(24/16)	15/32(32/16)	19/32(40/20)	19/32(40/20)	19/32(40/20)
Minimum Sheathing Thickness, inches(Panel Span Rating) Exposure C	7/16(24/16)	7/16(24/16)	15/32(32/16)	19/32(40/20)	19/32(40/20)	19/32(40/20)	19/32(40/20)	23/32(48/24)
Minimum Sheathing Thickness, inches(Panel Span Rating) Exposure D	15/32(32/16)	19/32(40/20)	19/32(40/20)	19/32(40/20)	19/32(40/20)	19/32(40/20)	23/32(48/24)	23/32(48/24)

ROOF SHEATHING NAILING PATTERN

Rafter/Truss Spacing24 in. o.c.	WIND SPEED															
	115 mph		120 mph		130 mph		140 mph		150 mph		160 mph		170 mph		180 mph	
	E	F	E	F	E	F	E	F	E	F	E	F	E	F	E	F
Exposure B																
Rafter/Truss SG = 0.42	6	6	6	6	6	6	6	6	6	6	4	4	4	4	4	4
Rafter/Truss SG = 0.49	6	12	6	12	6	6	6	6	6	6	6	6	6	6	6	6
Exposure C																
Rafter/Truss SG = 0.42	6	6	6	6	6	6	4	4	4	4	4	4	3	3	3	3
Rafter/Truss SG = 0.49	6	6	6	6	6	6	6	6	6	6	6	6	4	4	4	4
Exposure D																
Rafter/Truss SG = 0.42	6	6	6	6	4	4	4	4	4	4	3	3	3	3	3	3
Rafter/Truss SG = 0.49	6	6	6	6	6	6	6	6	4	4	4	4	4	4	4	4

E = NAIL SPACING ALONG PANEL EDGES (INCHES)
F = NAIL SPACING ALONG INTERMEDIATE SUPPORTS IN THE PANEL FIELD (INCHES)
A.FOR SHEATHING LOCATED A MINIMUM OF 4 FEET FROM THE PERIMETER EDGE OF THE ROOF, INCLUDING 4 FEET ON EACH SIDE OF RIDGES AND HIPS, NAIL SPACING IS PERMITTED TO BE 6 INCHES ON CENTER ALONG PANEL EDGES AND 6 INCHES ON CENTER ALONG INTERMEDIATE SUPPORTS IN THE PANEL FIELD.
B.WHERE RAFTER/TRUSS SPACING IS LESS THAN 24 INCHES ON CENTER, ROOF SHEATHING FASTENING IS PERMITTED TO BE IN ACCORDANCE WITH THE AWC WFCM OR THE AWC NDS.

NOTE:
FOR SHEATHING WITH THICKNESS LESS THAN OR EQUAL TO 15/32", SHEATHING SHALL BE FASTENED WITH ASTM F1667 RSRS-01 (2-3/8"X0.113") NAILS.

FOR SHEATHING WITH THICKNESS GREATER THAN 15/32", SHEATHING SHALL BE FASTENED WITH ASTM F1667 RSRS-04 (3"X0.120") NAILS.

UPLIFT HANGER SCHEDULE

TRUSS LABEL	UPLIFTS FORCES GREATER THAN 1000 LBS	HANGER TIE DOWN WOOD CONNECTION	HANGER TIE DOWN CONCRETE CONNECTION
1	1000 - 1195	HTS20	HETA20
2	1196 - 2390	(2) HTS20	(2) HETA20
3	2391 - 3685	(4) HTS20	HGT-2
4	3686 - 5420	(4) HTS20	HGT-3
5	5421 - 8000	(4) HTS20 + (4) MST136	HGT-3

NOTE: ALL HURRICANE STRAPPING SIMPSON STRONG-TIE UNLESS OTHERWISE NOTED.

LEGEND

B-#
TIE BEAM
WALL WITH TYPICAL BEAM
BEARING WALLS

FILLED CELL LEGEND

8" REINFORCED MASONRY BLOCK WITH (1) - #5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
12" REINFORCED MASONRY BLOCK WITH (1)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
16" REINFORCED MASONRY BLOCK WITH (2)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE
8" - 45° REINFORCED MASONRY CORNER BLOCK WITH (2)-#5 VERTICAL REBAR FILLED WITH A MINIMUM OF 3000 P.S.I. CONCRETE - (BUTTERFLY TYPE)

CONNECTION WOOD FRAME WALL / WOOD BEAM

UPLIFT (LBS)	SIMPSON CONNECTOR	CONNECTION SUPPORT MEMBER	CONNECTION TRUSS
0-1,310	HTS20	(12) .148"X1-1/2" NAILS	(12) .148"X 1-1/2" NAILS
1,310-2,620	(2) HTS20	EACH STRAP (12) .148"X1-1/2" NAILS	EACH STRAP (12) .148" X 1-1/2" NAILS
2,620-5,240	(4) HTS20	EACH STRAP (12) .148"X1-1/2" NAILS	EACH STRAP (12) .148" X 1-1/2" NAILS
0-1,640	MSTA24	EACH STRAP (9) .148"X1-1/2" NAILS	EACH STRAP (9) .148" X 1-1/2" NAILS
1,640-3,280	(2) MSTA24	EACH STRAP (9) .148"X1-1/2" NAILS	EACH STRAP (9) .148" X 1-1/2" NAILS
3,280-6,560	(4) MSTA24	EACH STRAP (9) .148"X1-1/2" NAILS	EACH STRAP (9) .148" X 1-1/2" NAILS
0-5,070	MST148	EACH STRAP (24) .148"X1-1/2" NAILS	EACH STRAP (24) .148" X 1-1/2" NAILS
5,070-10,140	(2) MST148	EACH STRAP (24) .148"X1-1/2" NAILS	EACH STRAP (24) .148" X 1-1/2" NAILS
0-1,705	CS16 (13"MM LENGTH PER COMPONENT)	EACH STRAP (24) .148"X1-1/2" NAILS	EACH STRAP (24) .148" X 1-1/2" NAILS

WOOD ROOF TRUSS TIE DOWN SCHEDULE

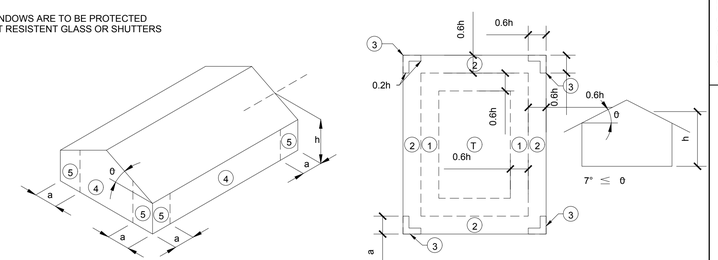
UPLIFT (LBS)	SIMPSON CONNECTOR	CONNECTION CONCRETE	CONNECTION TRUSS
0-1,810	HETA20	STRAP EMBEDDED CONCRETE	(9) .148" X 1-1/2" NAILS
0-2,120	HHETA20	STRAP EMBEDDED CONCRETE	(10) .148" X 1-1/2" NAILS
0-2,480	DETAL20	STRAP EMBEDDED CONCRETE	(18) .148" X 1-1/2" NAILS
0-4,725	FGTR	(2) 1/2" X 5" TITEN HD	(18) 1/4" X 3" SDS
0-8,885	(2) FGTR	(4) 1/2" X 5" TITEN HD	(36) 1/4" X 3" SDS
0-10,690	HGT-2	(2) 3/4" EPOXY SET ROD 6" EMBEDMENT	(16) .148" X 3" NAILS
0-10,790	HGT-3	(2) 3/4" EPOXY SET ROD 6" EMBEDMENT	(16) .148" X 3" NAILS
0-11,455	HGT-4	(2) 3/4" EPOXY SET ROD 6" EMBEDMENT	(16) .148" X 3" NAILS
0-9,920	HG98 3 PLY OR MORE TRUSS	7/8" EPOXY SET ROD 9" EMBEDMENT	(3) 7/8" CARRIAGE BOLTS
0-13,335	HD12	1-1/8" EPOXY SET ROD 12" EMBEDMENT	(4) 1" CARRIAGE BOLTS

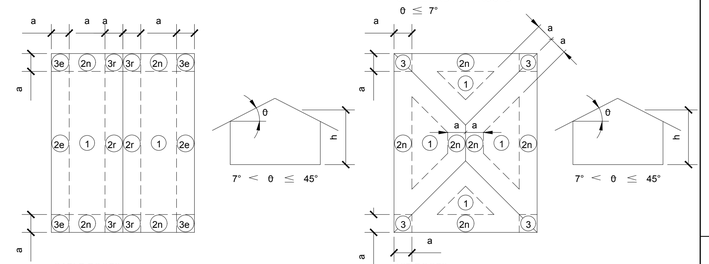
COMPONENT AND CLADDING DESIGN PRESSURES Vult = 170 MPH ULTIMATE DESIGN WIND SPEED COMPONENT AND CLADDING (BASED ON Vult) EXPOSURE C ULTIMATE DESIGN PRESSURES (LRFD) PSF

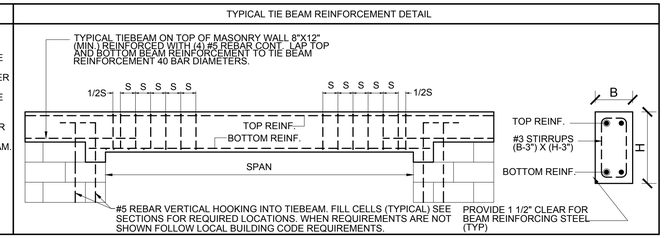
	ZONE	EFFECTIVE WIND AREA(ft2)	POSITIVE PRESSURE	NEGATIVE PRESSURE		ZONE	EFFECTIVE WIND AREA(ft2)	POSITIVE PRESSURE	NEGATIVE PRESSURE
GABLE ROOF 0 TO 7 DEGREES	1, 1'g	10	17.8	-69.6	HIP ROOF 7 TO 20 DEGREES	1	10	32.6	-73.2
	1, 1'g	20	16.7	-61.7		1	20	28.1	-73.2
	1, 1'g	50	15.1	-51.4		1	50	22.3	-56.6
	1, 1'g	100	14.0	-43.7		1	100	17.8	-43.7
	2	10	17.8	-91.8		2r	10	32.6	-95.5
	2	20	16.7	-81.8		2r	20	28.1	-86.1
	2	50	15.1	-68.5		2r	50	22.3	-73.5
	2	100	14.0	-58.5		2r	100	17.8	-64.0
	3	10	17.8	-125.2		2e, 3	10	32.6	-102.9
	3	20	16.7	-105.1		2e, 3	20	28.1	-92.5
	3	50	15.1	-78.4		2e, 3	50	22.3	-78.8
	3	100	14.0	-58.5		2e, 3	100	17.8	-68.5
GABLE ROOF > 7 TO 20 DEGREES	1, 2e	10	32.6	-80.6	HIP ROOF > 20 TO 27 DEGREES	1	10	32.6	-58.5
	1, 2e	20	28.1	-80.6		1	20	28.1	-51.8
	1, 2e	50	22.3	-49.3		1	50	22.3	-43.0
	1, 2e	100	17.8	-25.2		1	100	17.8	-36.3
	2n, 2r, 3e	10	32.6	-117.7		2e, 2r, 3	10	32.6	-80.6
	2n, 2r, 3e	20	28.1	-101.6		2e, 2r, 3	20	28.1	-72.1
	2n, 2r, 3e	50	22.3	-80.1		2e, 2r, 3	50	22.3	-60.8
	2n, 2r, 3e	100	17.8	-64.0		2e, 2r, 3	100	17.8	-52.2
	3r	10	32.6	-139.9		1	10	31.1	-62.2
	3r	20	28.1	-119.8		1	20	27.2	-55.2
	3r	50	22.3	-93.2		1	50	21.7	-45.9
	3r	100	17.8	-73.2		1	100	17.8	-38.9
GABLE ROOF > 20 TO 27 DEGREES	1, 2e	10	32.6	-62.2	HIP ROOF > 27 TO 45 DEGREES	2e	10	31.1	-74.1
	1, 2e	20	28.1	-62.2		2e	20	27.2	-58.5
	1, 2e	50	22.3	-52.9		2e	50	21.7	-36.3
	1, 2e	100	17.8	-45.5		2e	100	17.8	-36.3
	2n, 2r, 3e	10	32.6	-99.1		2r	10	31.1	-93.7
	2n, 2r, 3e	20	28.1	-86.9		2r	20	27.2	-78.7
	2n, 2r, 3e	50	22.3	-70.7		2r	50	21.7	-58.7
	2n, 2r, 3e	100	17.8	-58.5		2r	100	17.8	-43.7
	3r	10	32.6	-117.7		3	10	31.1	-100.0
	3r	20	28.1	-99.1		3	20	27.2	-75.2
	3r	50	22.3	-73.2		3	50	21.7	-43.7
	3r	100	17.8	-73.2		3	100	17.8	-43.7
GABLE ROOF > 27 TO 45 DEGREES	1, 2e, 2r	10	39.9	-73.2	WALLS	4	10	43.7	-47.3
	1, 2e, 2r	20	35.6	-62.2		4	20	41.7	-45.4
	1, 2e, 2r	50	29.5	-47.3		4	50	39.1	-42.8
	1, 2e, 2r	100	25.2	-36.3		4	100	37.1	-40.9
	2n, 3r	10	39.9	-80.6		4	500	32.6	-36.3
	2n, 3r	20	35.6	-72.1		5	10	43.7	-58.5
	2n, 3r	50	29.5	-60.8		5	20	41.7	-54.5
	2n, 3r	100	25.2	-52.2		5	50	39.1	-49.4
	3e	10	39.9	-99.1		5	100	37.1	-45.4
	3e	20	35.6	-88.1		5	500	32.6	-36.3
	3e	50	29.5	-73.2	GARAGE DOORS	5	9'X7'	38.6	-43.7
	3e	100	25.2	-62.2		5	16'X7'	37.0	-41.2

WIND LOAD REQUIREMENTS
1. THE STRUCTURAL SYSTEMS FOR THE DRAWINGS PRESENTED WERE DESIGNED PER THE LOADING PRESENTED IN THE FLORIDA BUILDING CODE 2020 7TH EDITION. THE DESIGN WIND SPEED IS (Vult = 170 MPH.) AND (Vasd = 132 MPH.)
2. IMPORTANCE FACTOR Iw = 1.0 OF THE FLORIDA BUILDING CODE 2020 7TH EDITION.
3. EXPOSURE CATEGORY C. OF THE FLORIDA BUILDING CODE
4. INTERNAL PRESSURE COEFFICIENT (ASCE 7-16) +.18/-.18 ENCLOSED BUILDING OPENINGS ARE PROTECTED FROM FLYING DEBRIS WITH IMPACT GLASS AND/OR SHUTTERS.

NOTE: ALL DOORS AND WINDOWS ARE TO BE PROTECTED WITH A APPROVED IMPACT RESISTENT GLASS OR SHUTTERS





DEEP BEAM NOTE	TYPICAL TIE BEAM REINFORCEMENT DETAIL
MOST BEAM DESIGNATIONS CALL FOR A 16" DEEP BEAM TO BE CAST MONOLITHICALLY WITH THE TIE BEAM ON TOP OF THE MASONRY WALL FOR OPENINGS REQUIRING A DEEPER HEADER A DEEP BEAM MAY BE CAST. FOR BEAMS 20"-36" DEEP AN ADDITIONAL LAYER OF (2) #7 REBAR SHALL BE PLACED AT THE BOTTOM OF THE BEAM IN ADDITION TO THE RIENFORCING STEEL ALREADY SPECIFIED. FOR BEAMS 36"+54" DEEP (2) LAYERS OF (2) REINFORCING STEEL ALREADY SPECIFIED. FOR BEAMS 36"-54" DEEP (2) LAYERS OF (2) #7 REBAR SHALL BE PLACED EQUAL DISTANCES APART FROM THE SPECIFIED BEAM. IN ADDITION TO THE SPECIFICATIONS ALREADY REQUIRED	

NO.	REVISION	DATE

GENERAL NOTES

DATE: 09/10/23 | PROJ. NO.: 1685-52-02 | DWN BY: FAA

LICENSED THIS TRADE IN ACCORDANCE WITH SPECIFICATIONS CONTAINED TO LOCAL BUILDING CODE REQUIREMENTS

Derek Cronin
Digitally signed by Derek Cronin
Date: 2023.09.16 13:05:49 -04'00'

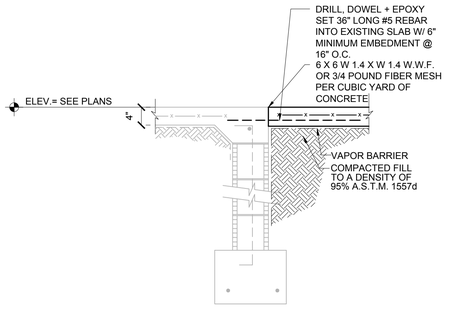
DEREK R. CRONIN
LICENSE NO. CCC1330820

CRONIN ENGINEERING, INC.
CERTIFICATE OF AUTHORIZATION NUMBER: 8597
6627 WILLOW PARK DRIVE
NAPLES, FL 34109
PHONE: (239) 593-2157 FAX: 593-8820

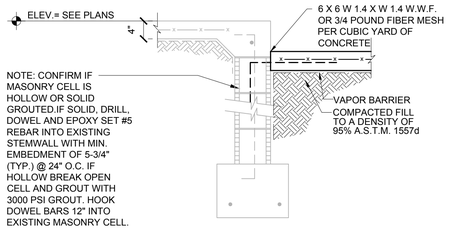
POPSTROKE PORT ST. LUCIE
SARASOTA, FL

MHK ARCHITECTURE & PLANNING
NAPLES, FL

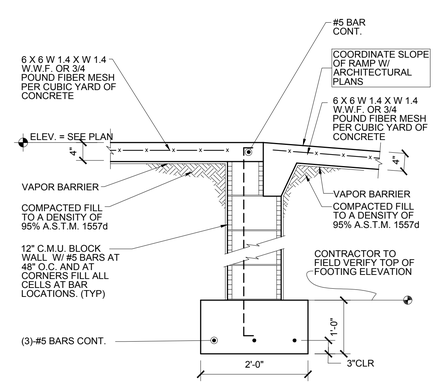
S-2



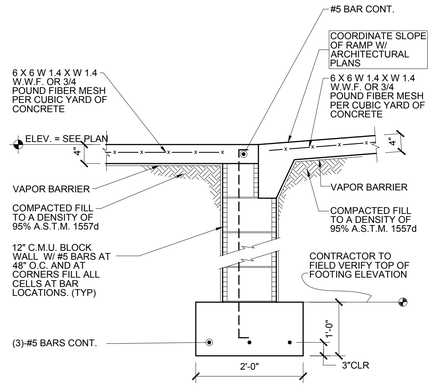
SECTION 5
SCALE: 3/4" = 1'-0"



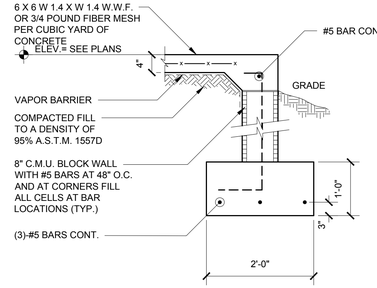
SECTION 4
SCALE: 3/4" = 1'-0"



SECTION 3
SCALE: 3/4" = 1'-0"



SECTION 2
SCALE: 3/4" = 1'-0"



SECTION 1
SCALE: 3/4" = 1'-0"

SECTIONS

DATE: 09/10/21 | PROJ. NO.: 1685-52-02 | DWN BY: FAA

NO.	REVISION	DATE

LICENSED PROFESSIONAL ENGINEER
STATE OF FLORIDA
Derek Cronin
signed by Derek Cronin
Date: 2021.09.16 14:06:59 -04'00'
DEREK R. CRONIN
LICENSE NO. 56705

CRONIN ENGINEERING, INC.
CERTIFICATE OF AUTHORIZATION NUMBER: 8597
6627 WILLOW PARK DRIVE
NAPLES, FL 34109
PHONE: (239) 593-2157 FAX: 593-8820

POPSTROKE PORT ST. LUCIE
SARASOTA, FL

MHK ARCHITECTURE & PLANNING
NAPLES, FL

S-3