

DESIGN CRITERIA

2023 FLORIDA BUILDING CODE (8TH EDITION)
ASCE7-16 AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14)
AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
ALLOWABLE STRENGTH DESIGN (ASD)(AISC 360-10) FOURTEENTH EDITION, 2010
AMERICAN WELDING SOCIETY D1.1
AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS

WINDSPEED

WIND - PARAMETERS

BASIC WIND SPEED	160MPH
IMPORTANCE FACTOR	1.0
EXPOSURE CLASS	C

SCOPE OF WORK

PROVIDE THE FOUNDATIONS FOR THE ADDITION OF PEMB FACILITY AT TODDVILLE TESTING LAB PER PROVIDED REACTIONS FROM PEMB COLUMNS AND SLAB ON GRADE FOR THE VEHICLE STORAGE.

STRUCTURAL DRAWING LIST

S01.00	STRUCTURAL GENERAL NOTES
S01.01	STRUCTURAL GENERAL NOTES
S02.00	FOUNDATION PLAN
S03.00	FOUNDATION DETAILS
S03.01	FOUNDATION DETAILS

GENERAL NOTES

SUBMITTALS: SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER PRIOR TO ANY FABRICATION OR CONSTRUCTION FOR ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING: CONCRETE REINFORCEMENT, EMBEDDED STEEL ITEMS, STRUCTURAL STEEL.

IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF THE REGISTERED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED.

NONSTRUCTURAL COMPONENTS: DESIGN, DETAILING AND ANCHORAGE OF ALL NONSTRUCTURAL COMPONENTS SHALL BE IN ACCORDANCE WITH ASCE 7-16 CHAPTER 13, AND THE PROJECT SPECIFICATIONS. NONSTRUCTURAL COMPONENTS DESIGNED BY OTHERS SHALL NOT INDUCE TORSIONAL LOADING INTO SUPPORTING STEEL STRUCTURAL MEMBERS WITHOUT ADDITIONAL BRACING OF THOSE MEMBERS TO ELIMINATE TORSIONAL FORCES. TORSIONAL BRACING SHALL BE DESIGNED BY THE NONSTRUCTURAL COMPONENT DESIGNER AND APPROVED BY THE ENGINEER.

CLADDING: CLADDING DESIGNED BY OTHERS SHALL BE SUPPORTED AT EACH STORY TO BE CONSISTENT WITH THE DESIGN OF THE BUILDING STRUCTURE. CLADDING DESIGNED BY OTHERS SHALL NOT INDUCE TORSIONAL LOADING INTO SUPPORTING STEEL STRUCTURAL MEMBERS WITHOUT ADDITIONAL BRACING OF THOSE MEMBERS TO ELIMINATE TORSIONAL FORCES, UNLESS OTHERWISE APPROVED BY THE ARCHITECT. TORSIONAL BRACING SHALL BE DESIGNED BY THE CLADDING DESIGNER AND APPROVED BY THE ENGINEER.

INSPECTION: SPECIAL INSPECTION SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY AS OUTLINED IN THE SPECIAL INSPECTION SCHEDULE AND AS INDICATED IN THE PROJECT SPECIFICATIONS. ALL PREPARED SOIL-BEARING SURFACES SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF REINFORCING STEEL. SOILS COMPACTION SHALL BE SUPERVISED BY AN APPROVED TESTING AGENCY OR GEOTECHNICAL ENGINEER.

SPECIAL CONDITIONS: CONTRACTOR SHALL VERIFY ALL LEVELS, DIMENSIONS, AND EXISTING CONDITIONS IN THE FIELD BEFORE PROCEEDING. CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR FIELD CHANGES PRIOR TO INSTALLATION OR FABRICATION. IN CASE OF DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE DRAWINGS, THE CONTRACTOR SHALL OBTAIN DIRECTION FROM THE ARCHITECT BEFORE PROCEEDING. DIMENSIONS NOTED AS PLUS OR MINUS (+/-) INDICATE UNVERIFIED DIMENSIONS AND ARE APPROXIMATE. NOTIFY ARCHITECT IMMEDIATELY OF CONFLICTS OR EXCESSIVE VARIATIONS FROM INDICATED DIMENSIONS. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS--DO NOT SCALE DRAWINGS. DIMENSIONS OF EXISTING CONDITIONS MAY BE BASED ON RECORD DRAWINGS AND ARE TO BE FIELD-VERIFIED BY THE CONTRACTOR.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING OF ALL STRUCTURAL MEMBERS, EXISTING CONSTRUCTION AND SOIL EXCAVATIONS, AS REQUIRED, AND IN A MANNER SUITABLE TO THE WORK SEQUENCE. TEMPORARY SHORING AND BRACING SHALL NOT BE REMOVED UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS AND MATERIALS HAVE ACHIEVED DESIGN STRENGTH. NO REINFORCING BARS IN EXISTING CONSTRUCTION SHALL BE CUT UNLESS DIRECTED TO BY THE ARCHITECT OR AS SHOWN ON THE DRAWINGS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.

SOILS: EARTHWORK MATERIAL, BACKFILL AND COMPACTION SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT PROPOSED FOR THE SITE AT 301 MCCOLLOUGH DRIVE, SUITE 109, CHARLOTTE, NC 28262 BY NOVA ENGINEERING AND ENVIRONMENTAL INC. PROJECT NUMBER: 10705-2020040 DATED: NOVEMBER 12, 2020

ALL FOUNDATIONS SHALL BE DESIGNED FOR THE MIN. BEARING CAPACITY OF 2000 PSF PER GEOTECH REPORT.

USE VAPOR RETARDER OF MIN. 15 MIL THICK UNDER SLAB ON GRADE. SEE ACI 302 FOR PROCEDURE AND CAUTIONS FOR PLACEMENT.

REFER TO GEOTECHNICAL REPORT FOR MORE INFORMATION ON SLAB ON GRADE.

STRUCTURAL STEEL NOTES

REFERENCE SPECIFICATIONS

STRUCTURAL STEEL	AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
HIGH STRENGTH BOLTS USING	SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325

WELDING	AWS D1.1, TYPICAL AWS D1.3 FOR METAL DECK AND COLD-FORMED FRAMING AWS PREQUALIFIED JOINT DETAILS
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WELDER CERTIFICATION	AMERICAN WELDING SOCIETY (AWS)
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STRUCTURAL STEEL NOTES (CONT'D)

GENERAL STEEL CONTRACTOR AND STEEL SHALL BE AISC CERTIFIED.

STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 22 OF THE IBC. ALL MEMBERS ARE TO BE ERECTED WITH NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE PLANS. SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE WILL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ARCHITECT. BOLTED CONNECTIONS ARE TO BE OF HIGH STRENGTH ASTM A 325 BOLTS AS SHOWN, UNLESS NOTED OTHERWISE. A MINIMUM OF TWO BOLTS IS REQUIRED FOR ALL BEAM CONNECTIONS. ALTERNATIVE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ARCHITECT.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:		
WIDE FLANGE SECTIONS	ASTM A992	Fy = 50 KSI
SQUARE AND RECTANGULAR HSS	ASTM A500, GR B	Fy = 46 KSI
PIPE SECTIONS	ASTM A53, GR B	Fy = 35 KSI
CAP AND BASE PLATES	ASTM A36	Fy = 36 KSI
CONNECTION MATERIAL	ASTM A36	Fy = 36 KSI
STIFFENER PLATES	ASTM A36	Fy = 36 KSI
ANCHOR RODS	ASTM F1554, GR 36	Fy = 36 KSI
HIGH STRENGTH BOLTS (AISC 360-05 ASD)	A325 (3/4" Ø UNO)	Fv = 24 KSI
TWIST-OFF BOLT/NUT/WASHER ASSEMBLIES	ASTM F1852	
HEAVY HEX NUTS	ASTM A563	
WASHERS	ASTM F436	
HEADED WELDED STEEL STUDS	ASTM A108, TYPE B	
ELECTRODES FOR ARC WELDING	AWS 5.1, E70XX	

COLD-FORMED STEEL

1. COLD-FORMED STRUCTURAL STUDS SHALL CONFORM TO THE FOLLOWING STANDARDS:
ROLLED SECTIONS, CONNECTION MATERIAL, STIFFENER PLATES

18 GAUGE AND THINNER	ASTM A653, GR 33	Fy = 33 KSI
16 GAUGE AND THICKER	ASTM A653, GR 50	Fy = 50 KSI
CONNECTION MATERIAL (>3/16" THICK)	ASTM A36	Fy = 36 KSI
ANCHOR RODS	ASTM F1554, GR 36	Fy = 36 KSI
BOLTS	ASTM A307	Fy = 10 KSI
COATING - HOT DIPPED	ASTM A924, G60	
ELECTRO - PLATE	ASTM A591	
ALUMINUM - ZINC	ASTM A792, GR 40	
INSTALLATION	ASTM C955 AND ASTM C1007	
ELECTRODES FOR ARC WELDING	AWS D1.1, E70XX	

ANCHORS

POST-INSTALLED ANCHORS: PROVIDE POST-INSTALLED ANCHORS AS SPECIFIED IN THESE DRAWINGS.

USE OF ALTERNATE PRODUCTS, OR OF POST-INSTALLED ANCHORS AT LOCATIONS NOT SHOWN IN THESE DRAWINGS, IS SUBJECT TO THE APPROVAL OF THE ARCHITECT. SUBMIT PROPOSED ANCHORS TO THE ARCHITECT WITH AN ICC-ES REPORT. SUBMITTED ICC-ES REPORTS SHALL DEMONSTRATE THAT THE ANCHORS ARE SUITABLE FOR USE IN CRACKED CONCRETE. WHERE ANCHORS RESIST SEISMIC LOADS, SUBMITTED ICC-ES REPORTS SHALL DEMONSTRATE THAT THE ANCHORS ARE SUITABLE FOR THE RESISTANCE OF SEISMIC LOADS.

HEADED SHEAR STUDS AND DEFORMED BAR ANCHORS: ALL HEADED SHEAR STUDS SHALL CONFORM TO ASTM A 108 AND SHALL BE 3/4-INCH DIAMETER HEADED STUDS, UNLESS NOTED OTHERWISE. STUD LENGTHS AFTER WELD SHALL BE AS SHOWN ON THE DRAWINGS. DEFORMED BAR ANCHORS (DBA) SHALL CONFORM TO ASTM A 496 AND SHALL BE OF THE SIZE AND LENGTH SHOWN ON THE DRAWINGS. ALL STUDS AND DEFORMED BAR ANCHORS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER.

DISCLAIMER:
FOUNDATION DESIGN SHALL BE RE-VISITED UPON THE RECEIVAL OF PEMB DESIGN DRAWINGS AND CONFORM TO THE MECKLENBURG COUNTY'S CODE ENFORCEMENT GUIDELINES FOR PRE-ENGINEERED METAL BUILDINGS OPTION B.

REFER TO PEMB/BUTLER MANUFACTURING DRAWINGS (ANCHOR ROD PLAN - DETAILS) FOR BASE PLATE AND ANCHOR ROD INFORMATION.



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Project Name and Address
FPL - WALTON ICE MACHINE
1050 SE BRANDON COURT, PORT ST. LUCIE, FL 34952

Sheet Title
STRUCTURAL GENERAL NOTES

Project No. 230-83.00	Sheet
Date AS NOTED	S01.00
Scale AS NOTED	

CONCRETE

CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF CHAPTER 19 OF THE 2023 FLORIDA BUILDING CODE (8TH EDITION).

CONCRETE MIXES: CONCRETE MIXES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

f'c (psi)	TEST AGE (DAYS)	USE
3,500	28	SLAB-ON-GRADE, CURBS AND PADS
3,500	28	FOUNDATIONS

WATER-REDUCING ADMIXTURES MAY BE INCORPORATED IN CONCRETE MIX DESIGNS, BUT SHALL CONFORM TO ASTM C 494, AND BE USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. CaCl2 OR OTHER WATER-SOLUBLE CHLORIDE ADMIXTURES SHALL NOT BE USED.

WATER/CEMENT RATIO SHALL BE MEASURED BY WEIGHT AND SHALL BE BASED ON THE TOTAL CEMENTITIOUS MATERIAL. WATER/CEMENT RATIO AND WATER CONTENT SHALL BE DETERMINED BY THE SUPPLIER BASED ON STRENGTH REQUIREMENTS AND SHALL NOT EXCEED THE MAXIMUM WATER/CEMENT RATIO AND/OR WATER CONTENT IF SHOWN ABOVE OR IN THE PROJECT SPECIFICATIONS.

FIELD-MEASURED SLUMP SHALL CONFORM TO THE SUBMITTED CONCRETE MIX DESIGN. TOLERANCE OF SLUMP SHALL CONFORM TO ASTM C 94.

AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C 260 SHALL BE USED IN ALL CONCRETE MIXES FOR FLATWORK WHICH IS EXPOSED TO WEATHER. THE AMOUNT OF ENTRAINED AIR SHALL BE 5% +/- 1 1/2 % BY VOLUME. THE AMOUNT OF ENTRAINED AIR SHALL BE MEASURED IN THE FIELD AT THE DISCHARGE FROM THE TRUCK.

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR APPROVAL 2 WEEKS PRIOR TO PLACING ANY CONCRETE. THE MIX DESIGN SHALL BE IN CONFORMANCE WITH 2018 NC BUILDING CODE. THE SUBMITTAL SHALL INDICATE WHERE EACH CONCRETE MIX IS TO BE USED ON THE PROJECT, AS WELL AS THE MAXIMUM AGGREGATE SIZE OF EACH MIX. MAXIMUM AGGREGATE SIZE SHALL CONFORM TO THE PROJECT SPECIFICATIONS.

CURING: IF THE AIR TEMPERATURE WILL EXCEED 75 DEGREES F WITHIN 48 HOURS OF PLACING CONCRETE, A MOIST CURE SHALL BE APPLIED TO THE CONCRETE FOR A PERIOD OF 36 HOURS AFTER FINISHING CONCRETE SURFACES. REFER TO THE PROJECT SPECIFICATIONS FOR CURING REQUIREMENTS.

REINFORCING STEEL

DEFORMED BARS	ASTM A 615, GRADE 60
WELDED WIRE FABRIC (WWF)	ASTM A 185 (fy = 65,000 psi)
DEFORMED WIRE FABRIC (DWF)	ASTM A 497 (fy = 70,000 psi)

REINFORCING SHALL BE SUPPORTED AS SPECIFIED BY THE PROJECT SPECIFICATIONS AND THE CRSI MANUAL OF STANDARD PRACTICE, 27TH EDITION. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI STANDARD OF PRACTICE AS OUTLINED IN "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT", ACI 315.

LAP ALL REINFORCING BARS AS NOTED ON THE DRAWINGS. WHERE SPLICE LENGTH IS NOT SHOWN, USE TYPE Lb (Lbt FOR TOP BARS) SPLICE PER DEVELOPMENT AND SPLICE LENGTH SCHEDULE. MECHANICAL OR WELDED BUTT SPLICES SHALL BE USED SUBJECT TO ARCHITECT'S APPROVAL.

REINFORCING STEEL SHALL HAVE PROTECTION AS FOLLOWS, UNLESS NOTED OTHERWISE:

USE	COVER
BEAM STIRRUPS AND COLUMN TIES	1 1/2"
SLAB-ON-GRADE	MID-DEPTH
WALL BARS	INTERIOR FACES 3/4"
	EXPOSED TO EARTH OR WEATHER 1 1/2" (#5 AND SMALLER)
	2" (#6 AND LARGER)
	FOOTING
	BOTTOM BARS 3" (CAST AGAINST EARTH)
	TOP BARS 1 1/2"
	2" (#6 AND LARGER WHERE EXPOSED TO EARTH OR WEATHER)
	SIDE BARS 2"

WELDING OF REINFORCING, WHEN APPROVED BY THE ARCHITECT, SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES AND PREHEATED IN ACCORDANCE WITH AWS D1.4, REINFORCING STEEL WELDING CODE. WELDERS AND WELDING PROCEDURES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.4. MATERIALS SHALL CONFORM TO THE FOLLOWING:

REINFORCING BARS TO BE WELDED	ASTM A 706, GRADE 60 LOW ALLOY
WELDING ELECTRODES	E80XX

NON-SHRINK GROUT: BASE PLATE GROUT SHALL BE NON-SHRINK TYPE WITH MINIMUM f'c = 8,000 psi. ALL OTHER NON-SHRINK GROUT SHALL HAVE MINIMUM f'c = 5,000 psi.



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1 PROPOSED PEMB COLUMN BY OTHERS (TYP.)
2 TYP. SLAB EDGE. SEE DETAIL 2/S03.00.

1. FIELD VERIFY EXISTING CONDITIONS PRIOR TO SUBMISSION OF SHOP DRAWINGS.
2. TOP OF SLAB ELEVATION TO MATCH EXISTING FINISH FLOOR = BUILDING DATUM ELEVATION +0'-0."
3. SEE ARCHITECTURAL DRAWINGS FOR OVERALL BUILDING DIMENSIONS OF WALLS AND WALL OPENINGS.
4. SEE SHEET S03.00 FOR FOOTING SCHEDULE AND SHEET S03.01 FOR BASE PLATE INFORMATION.
5. CENTER FOOTINGS ON COLUMN LINES U.N.O.
6. TOP OF FOOTING ELEVATION AT COLUMNS = (-1'-6") BELOW FINISH FLOOR U.N.O.
7. SEE 6/S03.00 FOR TYPICAL METAL BUILDING ANCHOR BOLT DETAILS.
8. COORDINATE METAL BUILDING ANCHOR BOLT DIAMETERS, QUANTITY, LOCATIONS AND PLACEMENT WITH ANCHOR BOLT SETTING PLAN PREPARED BY METAL BUILDING MANUFACTURER.
9. CJ INDICATES CONSTRUCTION JOINT OR SAWED CONTRACTION JOINT IN SLAB ON GRADE - SEE DETAILS 3/S03.00.
10. LATERAL LOAD DESIGN IS CONTROLLED BY WIND.
11. THE CONTRACTOR SHALL PROVIDE SHORING AS REQUIRED TO MAINTAIN THE STABILITY OF EXISTING STRUCTURES DURING DEMOLITION AND CONSTRUCTION.
12. SUBMIT SHOP DRAWINGS FOR FOUNDATION REINFORCING BARS REVIEW AND APPROVAL.

SLAB ON GRADE DESIGNED FOR THE AXLE LOAD OF 50KIPS WITH
100PCI SUB-GRADE MODULUS OF REACTION AND W/
COMPRESSIVE STRENGTH OF CONCRETE 3500 PSI FOR THE
H550-700HDS FORK LIFT VEHICLE - NOT LOADED AND
PNEUMATIC TIRE TYPE ONLY.

FOUNDATION DESIGN SHALL BE RE-VISITED UPON THE RECEIVAL OF PEMB DESIGN DRAWINGS AND CONFORM TO THE MECKLENBURG COUNTY'S CODE ENFORCEMENT GUIDELINES FOR PRE-ENGINEERED METAL BUILDINGS OPTION B.

REFER TO PEMB/BUTLER MANUFACTURING DRAWINGS (ANCHOR ROD PLAN - DETAILS) FOR BASE PLATE AND ANCHOR ROD INFORMATION.

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FOUNDATION PLAN

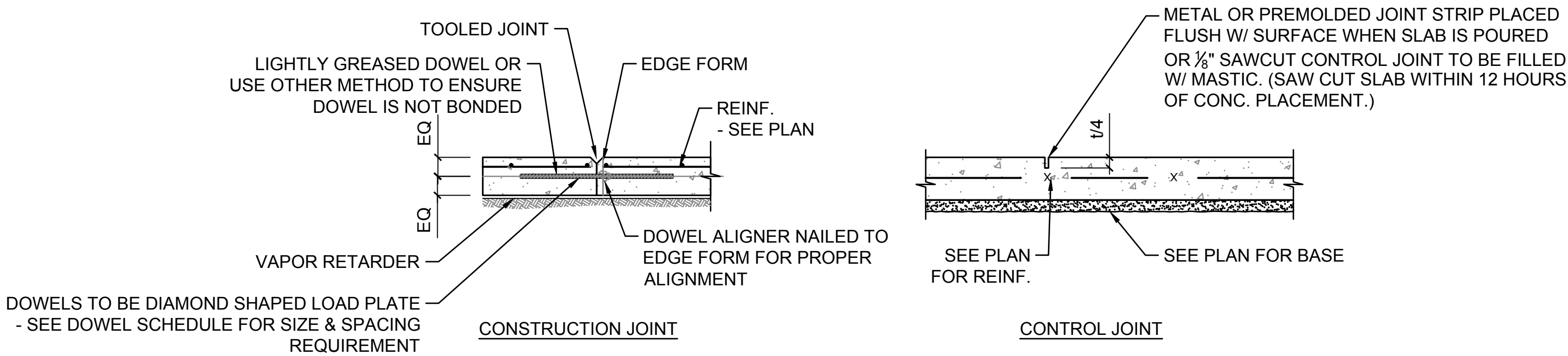
Project No. 230-83.00	Sheet S02.00
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COLUMN FOOTING SCHEDULE			
MARK	SIZE LENGTH X WIDTH X DEPTH	REINFORCEMENT	REMARK
F6.5	6'-6" X 6'-6" X 18"	7-#5'S EW T&B	
F8.0	8'-0" X 8'-0" X 18"	8-#6'S EW T&B	
NOTES: 1. SEE SHEET S03.02 FOR TYP. FOUNDATION DETAILS. 2. SEE SHEET S03.01 FOR TYP. BASE PLATE DETAILS. 3. SEE SHEET S02.00 FOR COLUMN PIER LOCATION & 5/S03.00 AND S03.03 FOR TYP. COLUMN PIER INFORMATION.			

COLUMN FOOTING SCHEDULE

SCALE
3/4" = 1'-0"

1



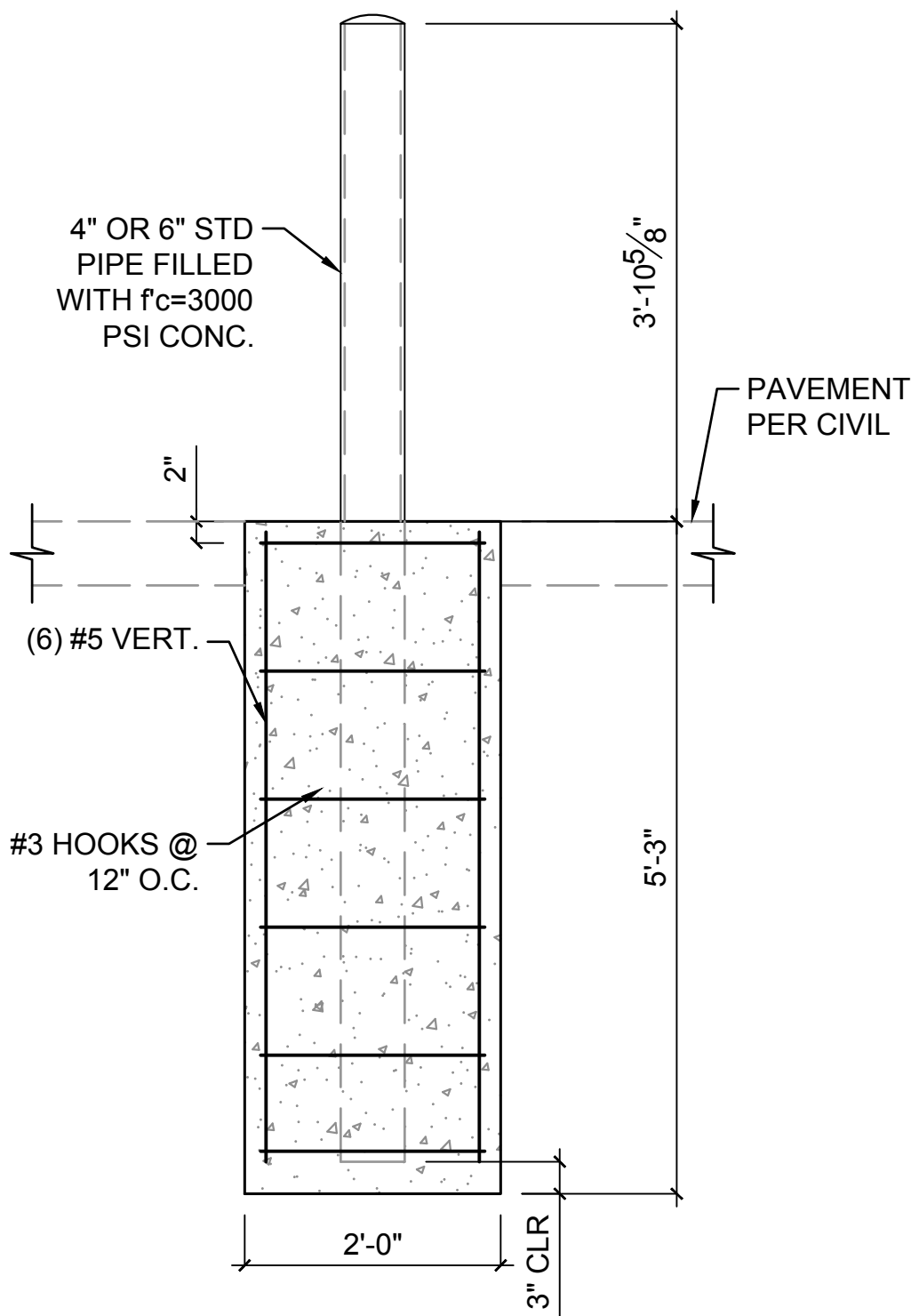
DOWEL SCHEDULE FOR DIAMOND-SHAPED LOAD PLATES								
SLAB DEPTH (IN.)	DOWEL DIMENSION (IN.)				DOWEL SPACING CENTER TO CENTER (IN.)			
	ROUND	SQUARE	RECTANGULAR	DIAMOND LOAD PLATE	ROUND	SQUARE	RECTANGULAR	DIAMOND LOAD PLATE
5" TO 6"	3/4 x 14	3/4 x 14	3/8 x 3 x 12	1/4 x 4 1/2 x 4 1/2	12	14	19	24

- NOTES:**
- CONSTRUCTION JOINT MAY REPLACE CONTROL JOINT.
 - REFER TO ARCHITECTURAL DETAILS FOR JOINT FILLER WHERE REQUIRED.
 - SEE PLAN FOR LOCATION OF JOINTS.
 - PROVIDE SUPPORT CHAIRS TO HOLD WWF AND/ OR REINFORCING IN POSITION DURING CONCRETE PLACEMENT.
 - DOWELS SHOULD BE PLACED NO CLOSER THAN 12 IN. FROM THE INTERSECTION OF ANY JOINTS.

TYP. SLAB JOINT DETAILS

SCALE
3/4" = 1'-0"

2

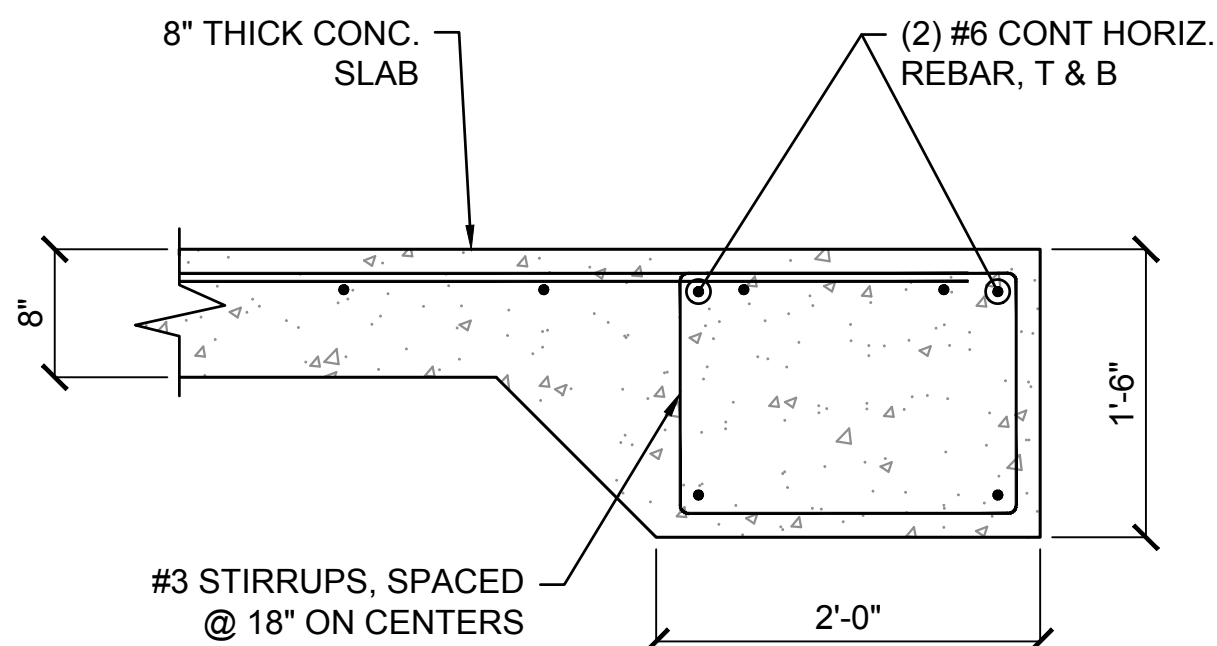


- NOTE:**
- WHERE BOLLARD IS ON TOP OF FOOTING, PROVIDE PL 1/2"x10"x10" BASE PLATE W/ (4) 3/8"Ø HILTI KWIK BOLT 3 EXPANSION ANCHOR (4 3/8" EMBED).

CONCRETE BOLLARD DETAIL

SCALE
3/4" = 1'-0"

3



MONOLITHIC FOOTER SECTION VIEW

SCALE
1/2" = 1'-0"

4



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